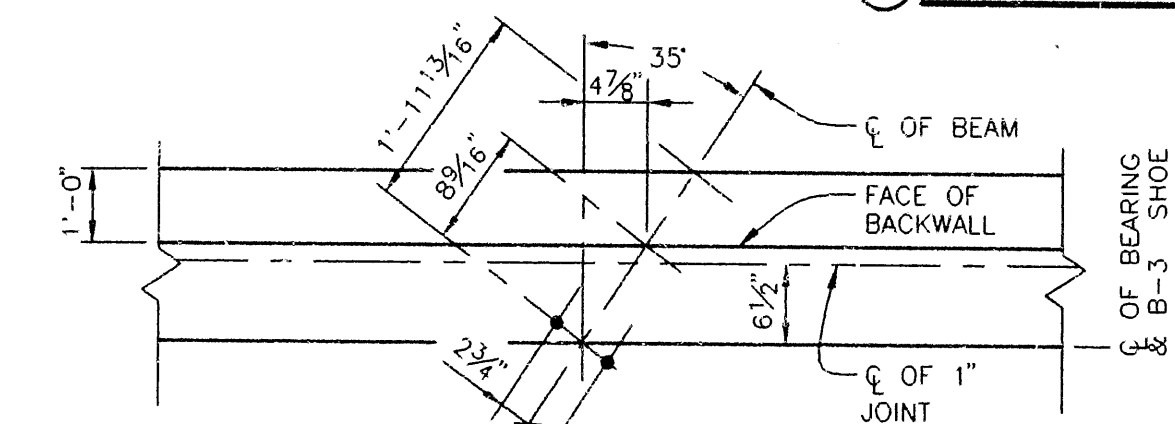
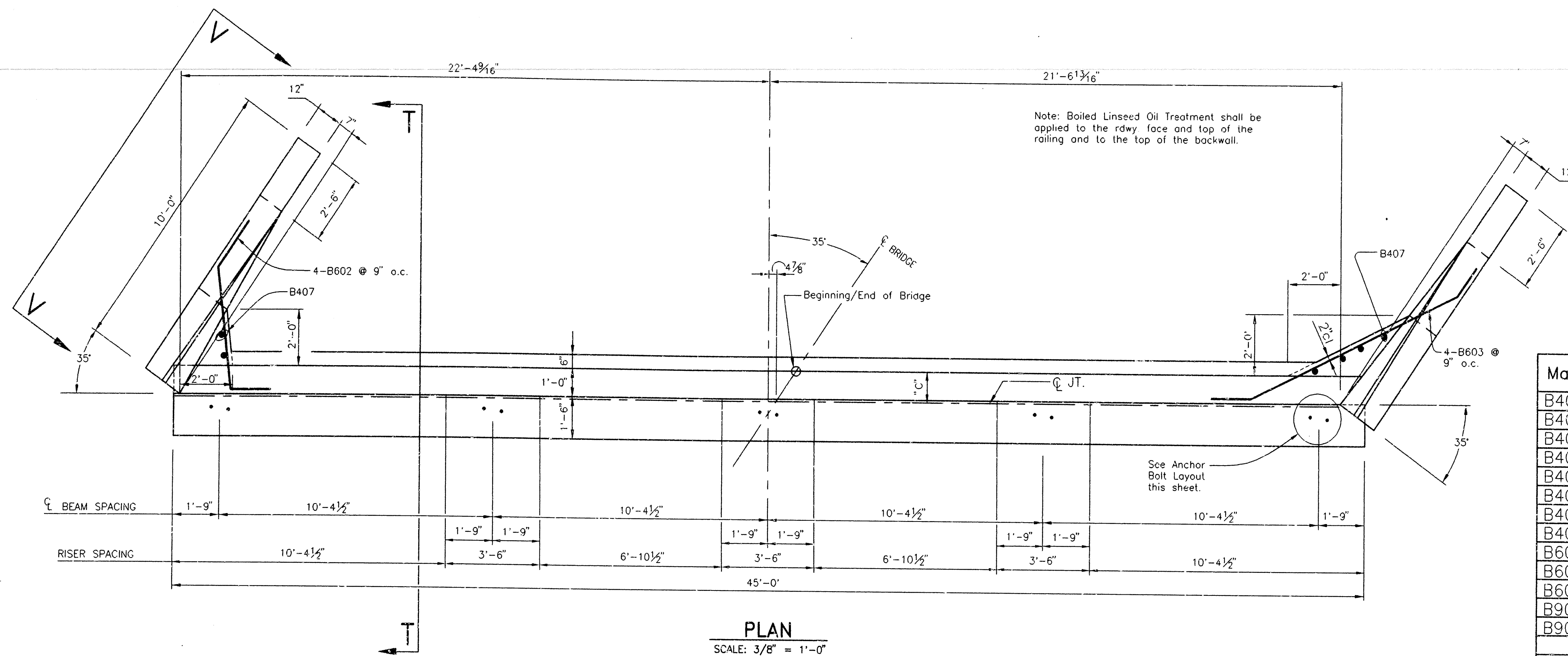








DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK			
						JOB NO.	SA6030	9
							4761	END BENTS
								36394



ANCHOR BOLT LAYOUT

SCALE: 3/8" = 1'-0"

BAR LIST

Mark	NO. REQ'D	Length	Pin Dia.
B401	9	6'-4"	2"
B402	64	7'-10"	2"
B403	20	9'-6"	2"
B404	82	6" + 1'-6"	STR
B405	41	3'-11"	2"
B406	16	23'-4"	STR
B407	6	6" + 1'-2"	STR
B408	8	22'-10"	STR
B601	8	6" + 1'-10"	STR
B602	4	6'-4"	4 1/2"
B603	4	10'-7"	4 1/2"
B901	4	47'-2"	9"
B902	4	44'-8"	STR
C401-C407	3 EA.	VARIES 8'-10 1/2" TO "p"	2"
C801	36	"1"	6"
F501	33	"n"	3 3/4"
F502	33	6'-8"	3 3/4"
R401	8	3'-11"	2"
R402	8	4'-0"	2"
R403	12	9'-8"	STR
R601	16	4'-5"	STR
R602	6	5'-0"	STR
W401	6	6" + 1'-2"	2"
W402	6	6" + 2'-5"	STR
W403-W407	ea.	Var. 3'-5" to 5'-5"	2"
W408	2	Var. 4'-6" to 6'-6"	STR
W412	ea.	Var. 4'-6" to 6'-6"	STR
W413	4	7'-11"	2"
W701	12	9'-8"	STR
W702	4	6'-0"	STR
W703	4	4'-6"	STR
W704	4	8'-4"	5 1/4"

Bending Diagrams  
(Dimensions are out to out of bars)

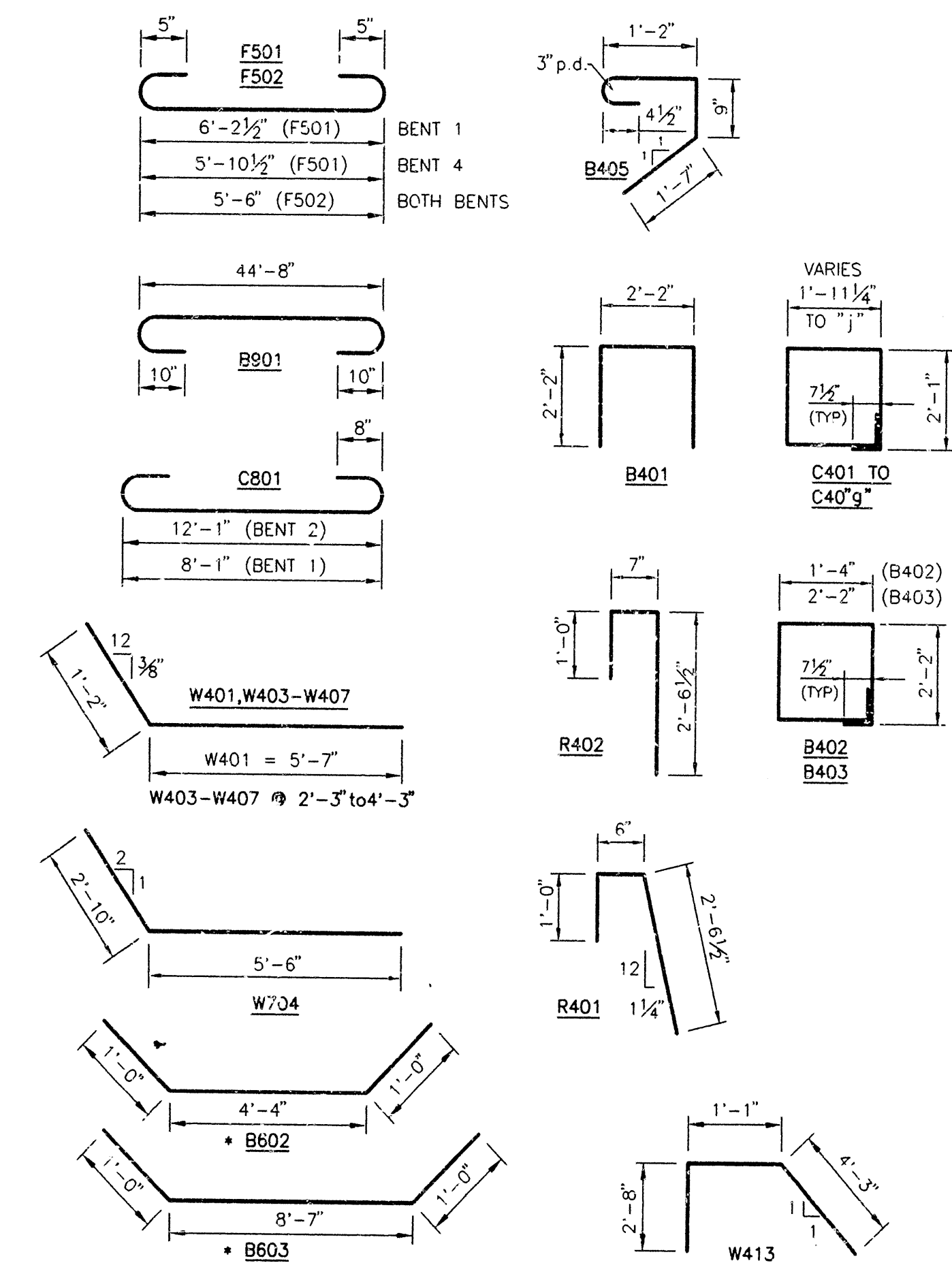


TABLE OF VARIABLES

Variable	FND BENT NUMBER
"a"	3'-5"
"b"	6'-4"
"c"	1'-0 1/2"
"d"	8'-6"
"e"	3'-2 1/2"
"f"	6'-8 1/2"
"g"	5'-0"
"h"	2'-9 1/4"
"i"	13'-11"
"j"	35'-6"
"k"	7'-4 1/2"
"l"	10'-8"
"m"	438.42
"n"	435.00
"o"	422.00
"p"	438.06

Tabular Data By: JVR Date: 27 Dec 94  
Checked By: KLD Date: 27 Feb 95

ALL CONCRETE SHALL BE CLASS "S" AND BE POURED IN THE DRY. ALL EXPOSED CORNERS TO BE CHAMFERED 3/4" UNLESS OTHERWISE NOTED.

ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 OR A617, GRADE 60.

BACKWALL SHALL NOT BE POURED BEFORE BEAMS ARE IN PLACE.

STRUCTURAL STEEL IN END BENTS SHALL BE ASTM A588 AND SHALL BE PAID FOR AS "STRUCTURAL STEEL IN BEAM BEAMS (A588)".

IF ANCHOR BOLTS ARE DRILLED INTO CAP, TOP REINFORCING BARS SHALL BE PROPERLY PLACED TO AVOID DAMAGE.

FOR ADDITIONAL INFORMATION, SEE LAYOUT.

DETAILS OF END BENT No. 1 & 4  
FOURCHE CREEK AT LAWSON ROAD  
PULASKI COUNTY  
CO. ROUTE 26 SEC.  
ARKANSAS STATE HIGHWAY COMMISSION

DRAWN BY: C.H. DATE: 22 FEB 95  
CHECKED BY: K.J. DATE: 27 FEB 95  
DESIGNED BY: T.R. DATE: 22 FEB 95

SCALE: AS NOTED  
FILENAME: 33513.DWG

BRIDGE NO. 4761

DRAWING NO. 36394

DATE: JUL 31 1995

SECTION A-A  
SCALE: 3/8" = 1'-0"

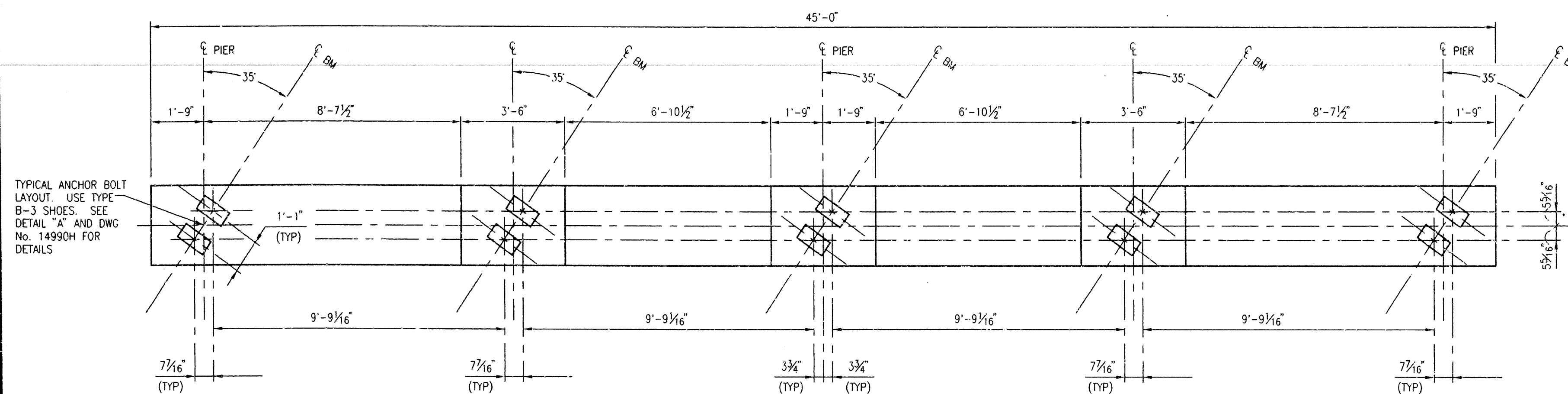
SECTION B-B  
SCALE: 3/8" = 1'-0"



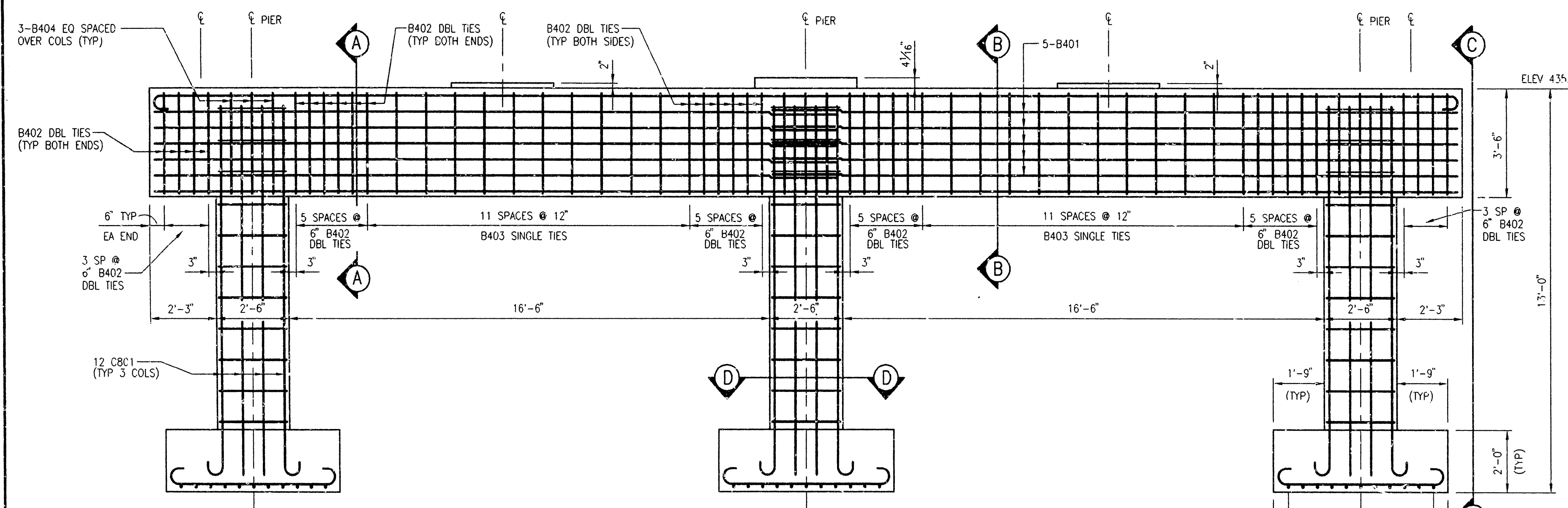




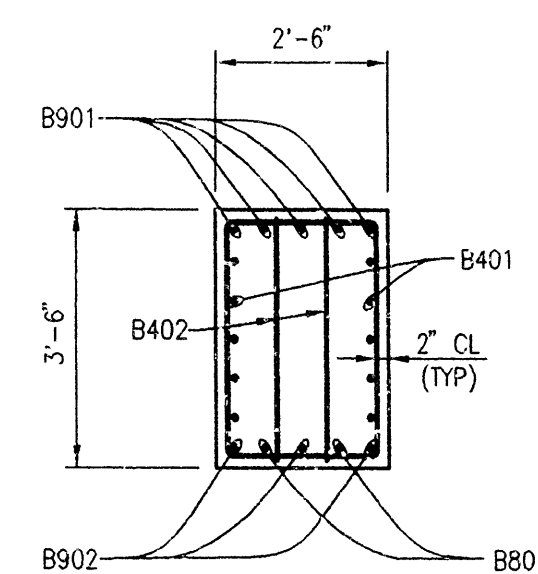
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				6	ARK			
				JOB NO.		SA6030	11	39
① 4761 INTERMEDIATE BENTS 36396								



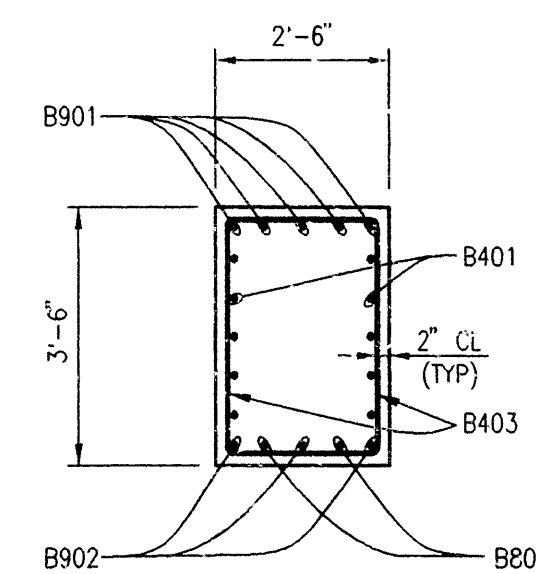
PLAN  
SCALE: 3/8" = 1'-0"



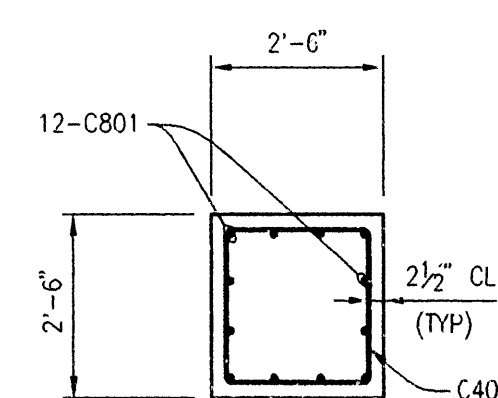
ELEVATION  
SCALE: 3/8" = 1'-0"  
BENT 2 & 3



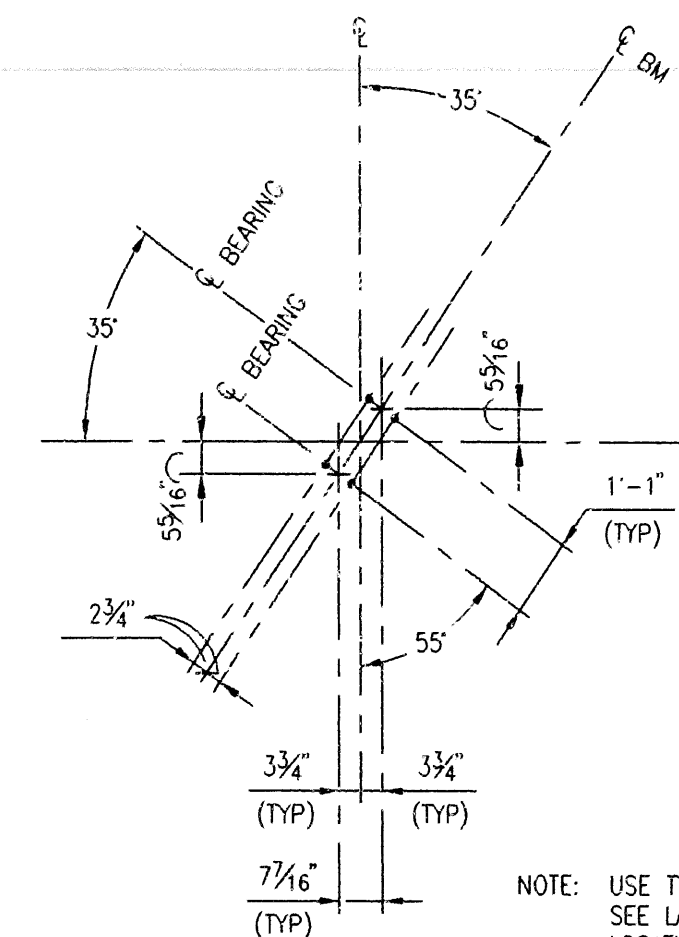
SECTION A-A  
SCALE: 3/8" = 1'-0"



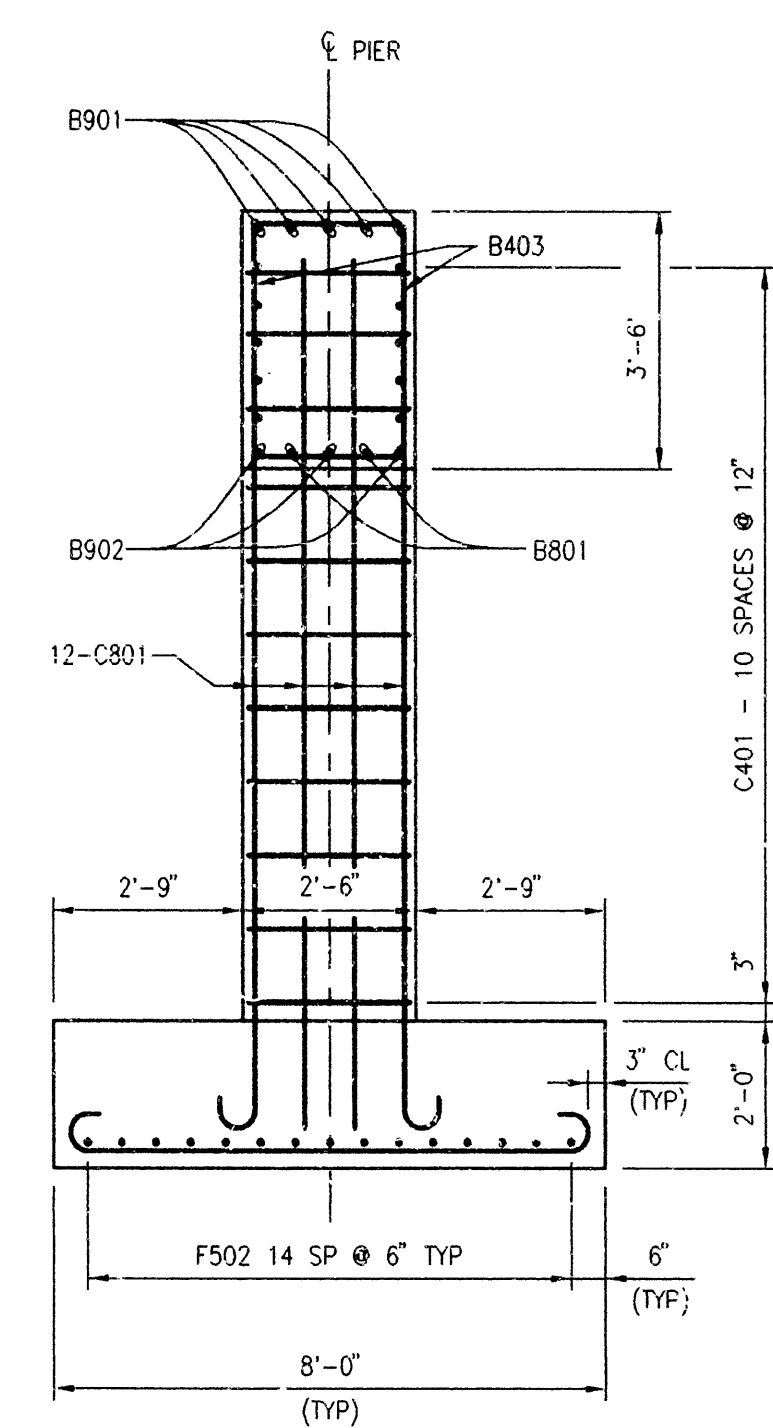
SECTION B-B  
SCALE: 3/8" = 1'-0"



SECTION D-D  
SCALE: 3/8" = 1'-0"



DETAIL A  
TYPICAL ANCHOR BOLT DIAGRAM  
SCALE: 3/8" = 1'-0"



SECTION C-C  
SCALE: 3/8" = 1'-0"

## BAR LIST

Mark	NO. REQ'D	Length	A	B	Pin Dia.	Bending Diagrams (Dimensions are out to out of bars)
B401	20	22'-10"	-	-	STR	
B402	64	9'-10"	1'-4"	3'-2"	2"	
B403	20	11'-5"	2'-2"	3'-2"	2"	
B404	9	8'-4"	2'-2"	3'-2"	2"	
B801	2	44'-8"	-	-	STR	
B901	5	47'-2"	44'-8"	10"	9"	
B902	3	44'-8"	-	-	STR	
C401	33	9'-2"	2'-1"	2'-1"	2"	
C801	36	12'-9"	11'-10"	8"	6"	
F501	33	6'-8"	5'-6"	5"	3 3/4"	
F502	45	8'-8"	7'-6"	5"	3 3/4"	

NOTE: DIMENSIONS ARE OUT TO OUT OF BARS.

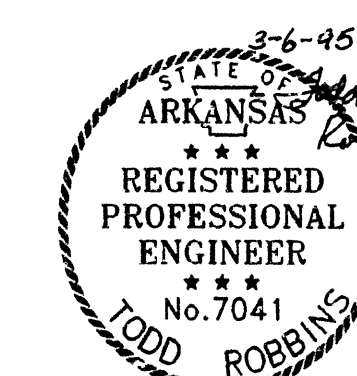
ALL CONCRETE SHALL BE CLASS "S" AND BE POURED IN THE DRY, ALL EXPOSED CORNERS TO BE CHAMFERED 3/4" UNLESS OTHERWISE NOTED.  
ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 OR A617, GRADE 60.  
BACKWALL SHALL NOT BE POURED BEFORE BEAMS ARE IN PLACE.  
STRUCTURAL STEEL IN END BENTS SHALL BE ASTM A588 AND SHALL BE PAID FOR AS "STRUCTURAL STEEL IN BEAM BEAMS (A588)".  
IF ANCHOR BOLTS ARE DRILLED INTO CAP, TOP REINFORCING BARS SHALL BE PROPERLY PLACED TO AVOID DAMAGE.  
FOR ADDITIONAL INFORMATION, SEE LAYOUT.

DETAILS OF INTERMEDIATE BENTS 2 & 3  
FOURCHE CREEK STR. & APPRS.  
PULASKI COUNTY  
CO. ROUTE 26 SEC.  
ARKANSAS STATE HIGHWAY COMMISSION

DRAWN BY: C.H. DATE: 22 FEB 95  
CHECKED BY: K.J. DATE: 27 FEB 95  
DESIGNED BY: T.R. DATE: 22 FEB 95

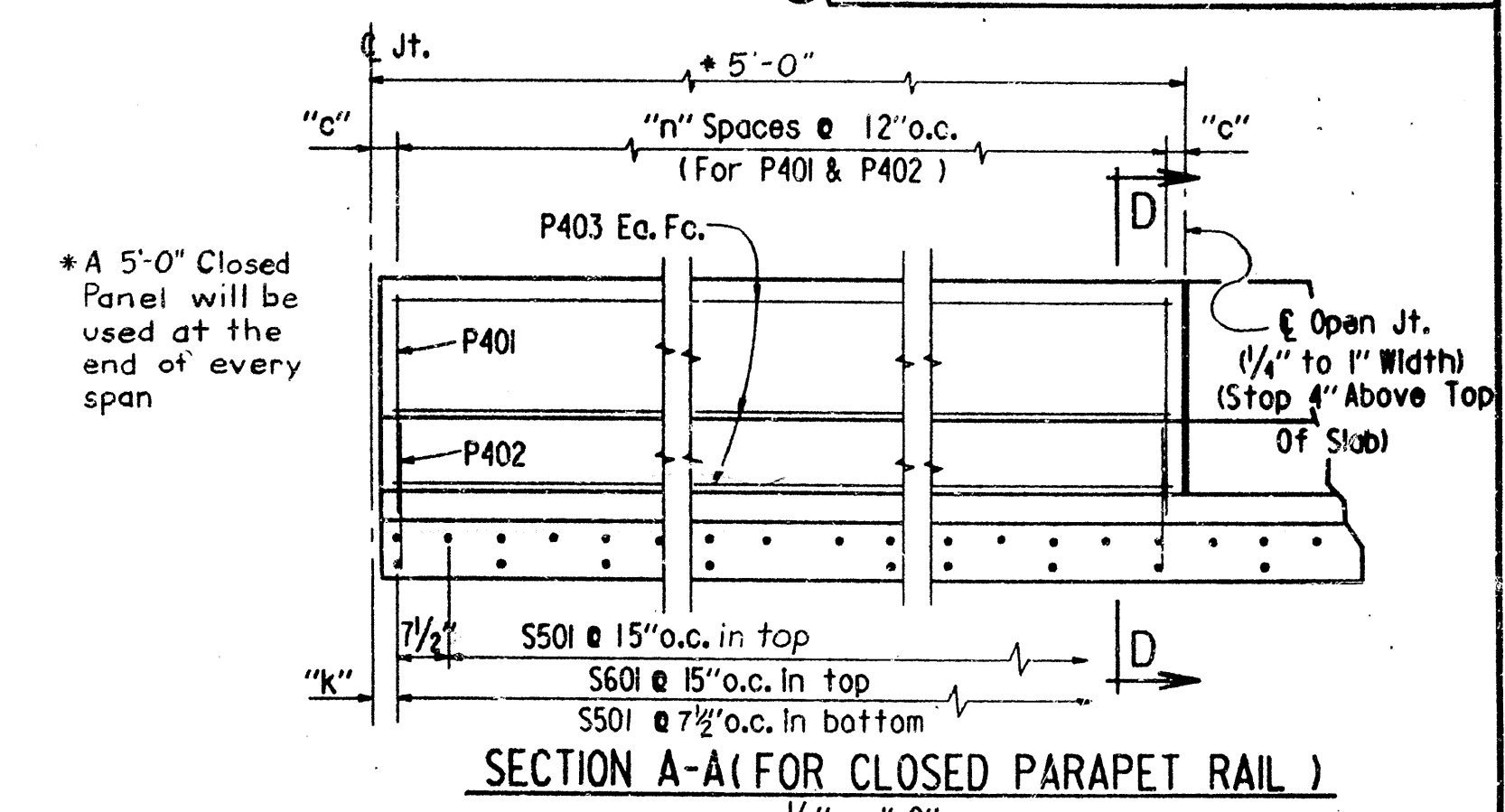
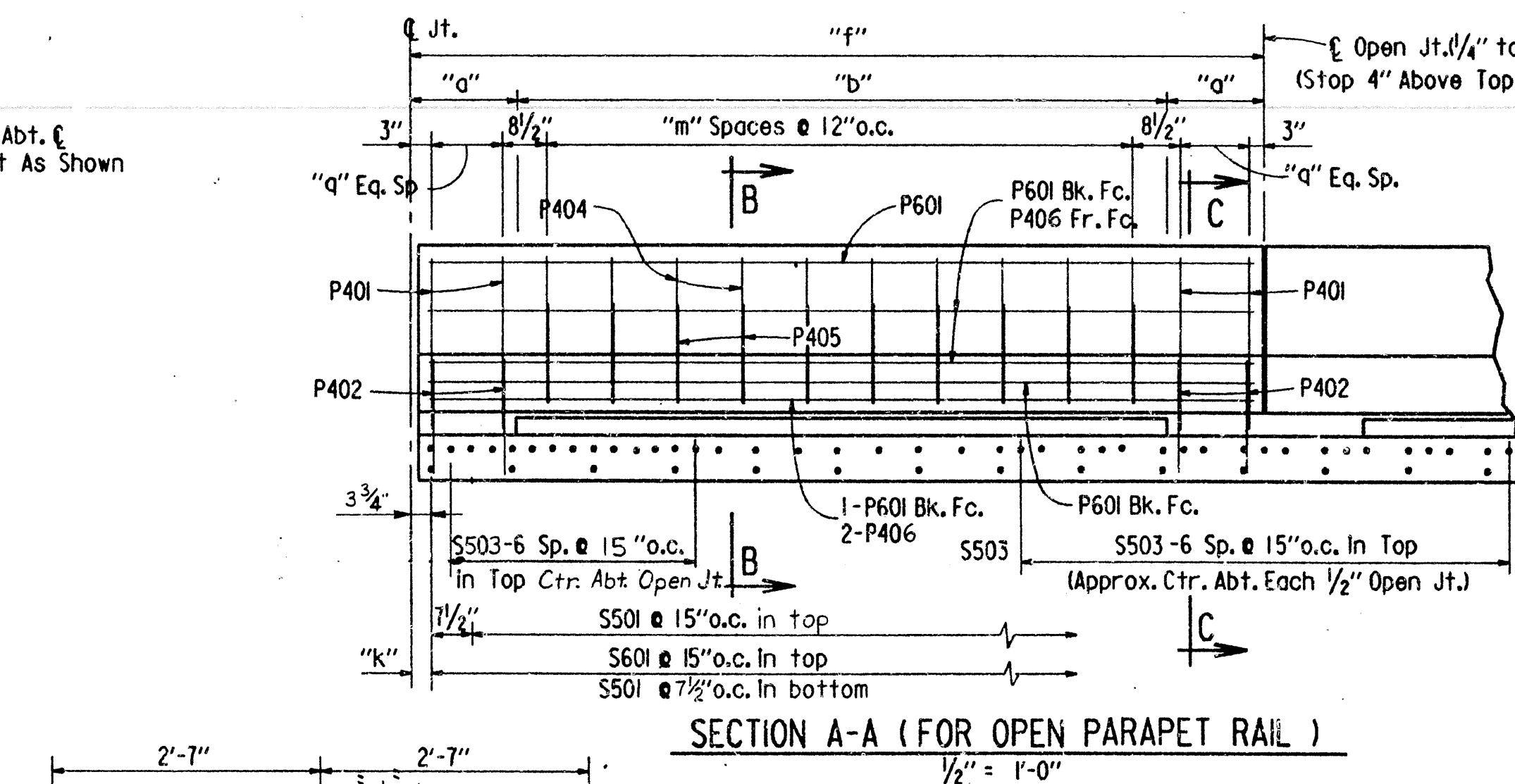
SCALE: 3/8" = 1'-0"  
FILENAME: 33515.DWG

BRIDGE NO. 4761 DRAWING NO. 36396

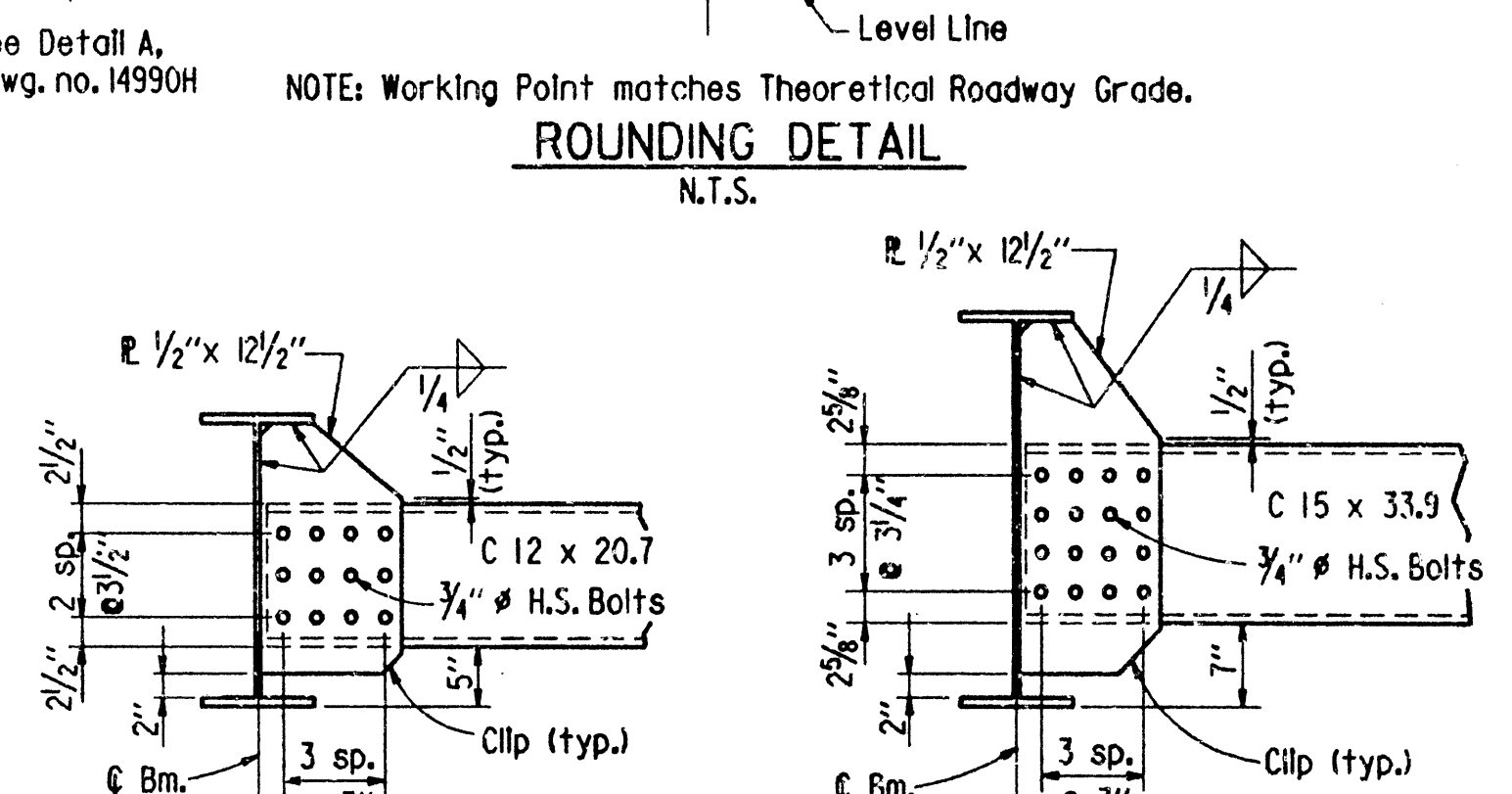




DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		SA6030	12	39
				① 4761	SPAN DETAILS			36397



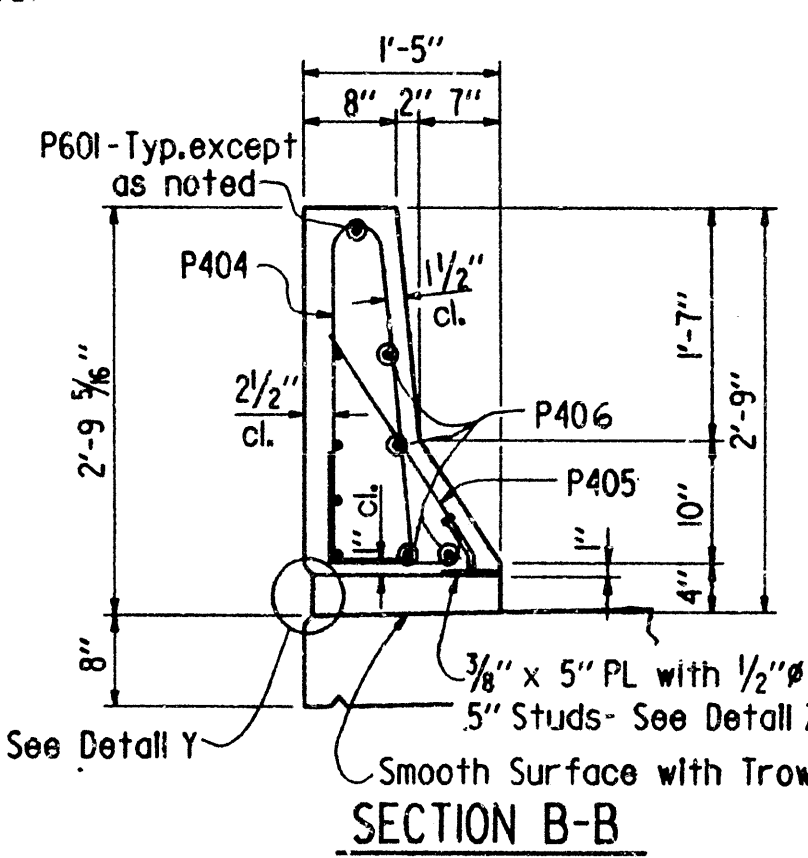
SECTION A-A (FOR OPEN PARAPET RAIL)



Note: Bolts in Diaphragm Connections shall be properly installed and tightened in accordance with Subsection 807.22 of the Standard Specifications.

<u>FOR W21, 24, 27</u> (Max. Diaphragm Spacing = 11'-0")	<u>FOR W30, 33, 36</u> (Max. Diaphragm Spacing = 16'-0")
---	---

DIAPHRAGM CONNECTIONS AT  
EXTERIOR BEAMS  
N.T.S.



1'-5"

8" 2" 7"

P601-Typ. except as noted

P401

1 1/2" cl.

2 1/2" cl.

P406

P402

10"

2'-9 5/8"

1'-7"

4"

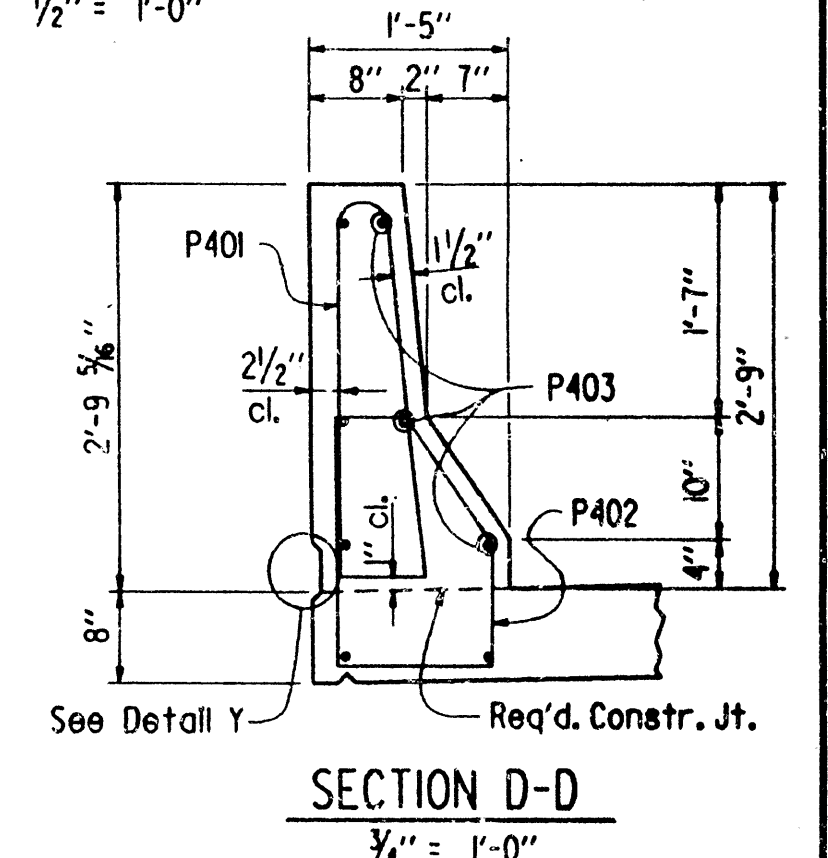
8"

See Detail Y

Req'd. Constr. Jt.

**SECTION C-C**

3/4" = 1'-0"



GENERAL NOTES

All Structural Steel shall be ASTM designation A588 unless otherwise noted and shall be paid for at the unit price per pound bid for "Structural Steel in Beam Spans (A588)". A588 steel shall not be painted. All exposed surfaces to be cleaned in accordance with Subsection 807.67(e) of the Standard Specifications. Structural steel completely embedded in concrete may be ASTM A36.

Beams and cover plates are considered main load carrying members and shall meet the longitudinal Charpy V-Notch test specified in Section 807.05.

Design Specifications: AASHTO 1992 with Interim Specifications

Live loading: HS20                      Method of Design: Load Factor

Dead Load:	Interior Beam	Exterior Beam
A. To W-Beam	857 plf + 1.3 (Wt./Ft. of W-Beam.)	661 plf + 1.3 (Wt./Ft. of W-Beam.)
B. To Composite Beam		
Open Parapets	316 plf *	316 plf *
Closed Parapets	329 plf *	329 plf *

Live Load: To each composite beam	1,546 wheels + impact	1,388 wheels + impact
-----------------------------------	-----------------------	-----------------------

\* includes 173 plf future wearing surface

Material Strength:	
Class (S)AE Concrete (N=8)	$f'_c = 4,000$ p.s.i.
Reinforcing Steel (A615 or A617)	$f_y = 60,000$ p.s.i.
Structural Steel (A36)	$F_y = 36,000$ p.s.i.
Structural Steel (A588)	$F_y = 50,000$ p.s.i.

For additional notes see Std. Drawing No. I4990H

Revised: 11-23-92. LDF

EXT. BEGIN	
Pt. 1/2 Pt.	
1/6 1/10	
1/6 1/2	
1/2 3/10	

DETAILS OF STANDARD  
 35'-90' COMPOSITE W-BEAM SPANS  
 CONCRETE PARAPET RAIL

36'-0" CL. RDWY.	0.02' /	PEAKED CROWN
ROUTE	SEC.	
ARKANSAS STATE	HIGHWAY	COMMISSION

LITTLE ROCK, ARK.

DRAWN BY: WMAJ      DATE: 11-28-90  
 CHECKED BY: AMS      DATE: 11-28-90      SCALE: As Shown  
 DESIGNED BY: \_\_\_\_\_

DESIGNED BY: \_\_\_\_\_ DATE: \_\_\_\_\_  
BRIDGE NO. 4761 DRAWING NO. 36397

I, 550, 300I, R600I6, RWME548, B36WBSPAN.STD

I, 550, 300I, R600I6, RWME548, B36WBSPAN.STD

[illegible]

\* For  $S \leq 40'$ ; Length =  $S - 6''$   
 For  $40' < S \leq 78'$ ; Length =  $S/2 + 7''$   
 For  $S > 78'$ ; Length =  $S/3 + 12''$

SPACING FOR  $\frac{7}{8}$ " STUD SHEAR  
CONNECTORS & DIAPHRAGMS

Note: Stud Shear Connectors shall be 4" long.  $\frac{3}{4}$ " $\phi$  Studs may be used in place of the  $\frac{1}{8}$ " $\phi$  Studs shown, at the ratio of 1.361 -  $\frac{3}{4}$ " $\phi$  Studs in place of one  $\frac{1}{8}$ " $\phi$  Stud.  $\frac{1}{8}$ " $\phi$  Stud will be used as basis for measurement of structural steel in shear connectors. Maximum Stud spacing = 24"

## TABLE OF VARIABLES

[illegible]

Tabular Data By: TVR Date: 2-21-95 Checked By: KLJ Date: 2-27-95 \* A 5'-0" Closed Parapet will be  
Used at the end of every span

**Note:** N.T.S.  
The surfaces of the  $\frac{3}{8}$ " Plates which will not be in contact with concrete shall be painted in accordance with Section 638, or as approved by the Engineer. Only one coat is required and shall be applied in the fabricator's shop. Painting will not be paid for directly, but will be considered subsidiary to Structural Steel or Class (S)AE Concrete-Bridge.

STATE OF  
ARKANSAS  
REGISTERED  
PROFESSIONAL  
ENGINEER  
No. 7041  
TODD ROBBINS

2-1-95  
J.R.

DEAD LOAD DEFLECTION

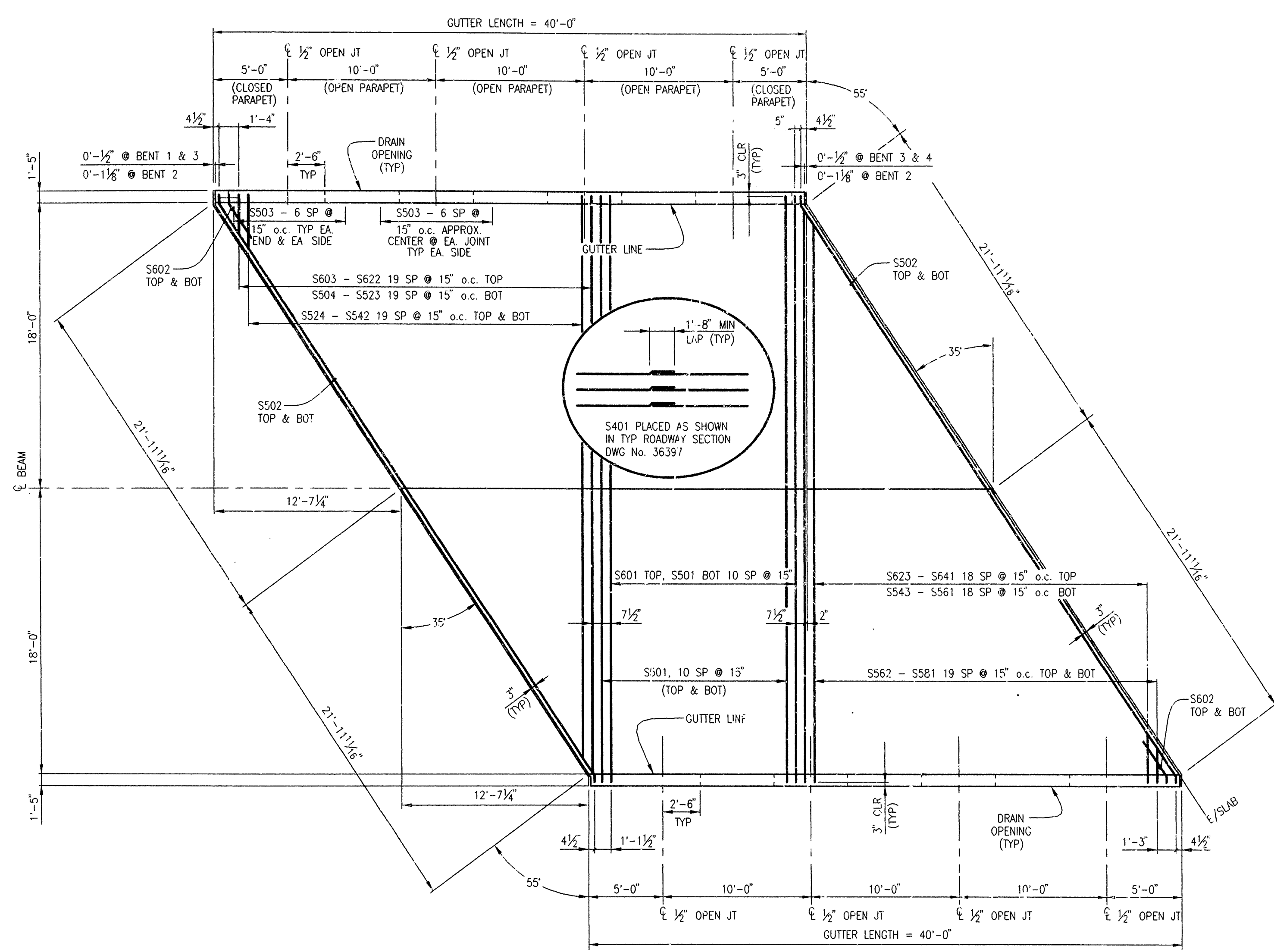
Span length Load No.	Loading	Lo
		Int. Beam

### DEAD LOAD DEFLECTIONS

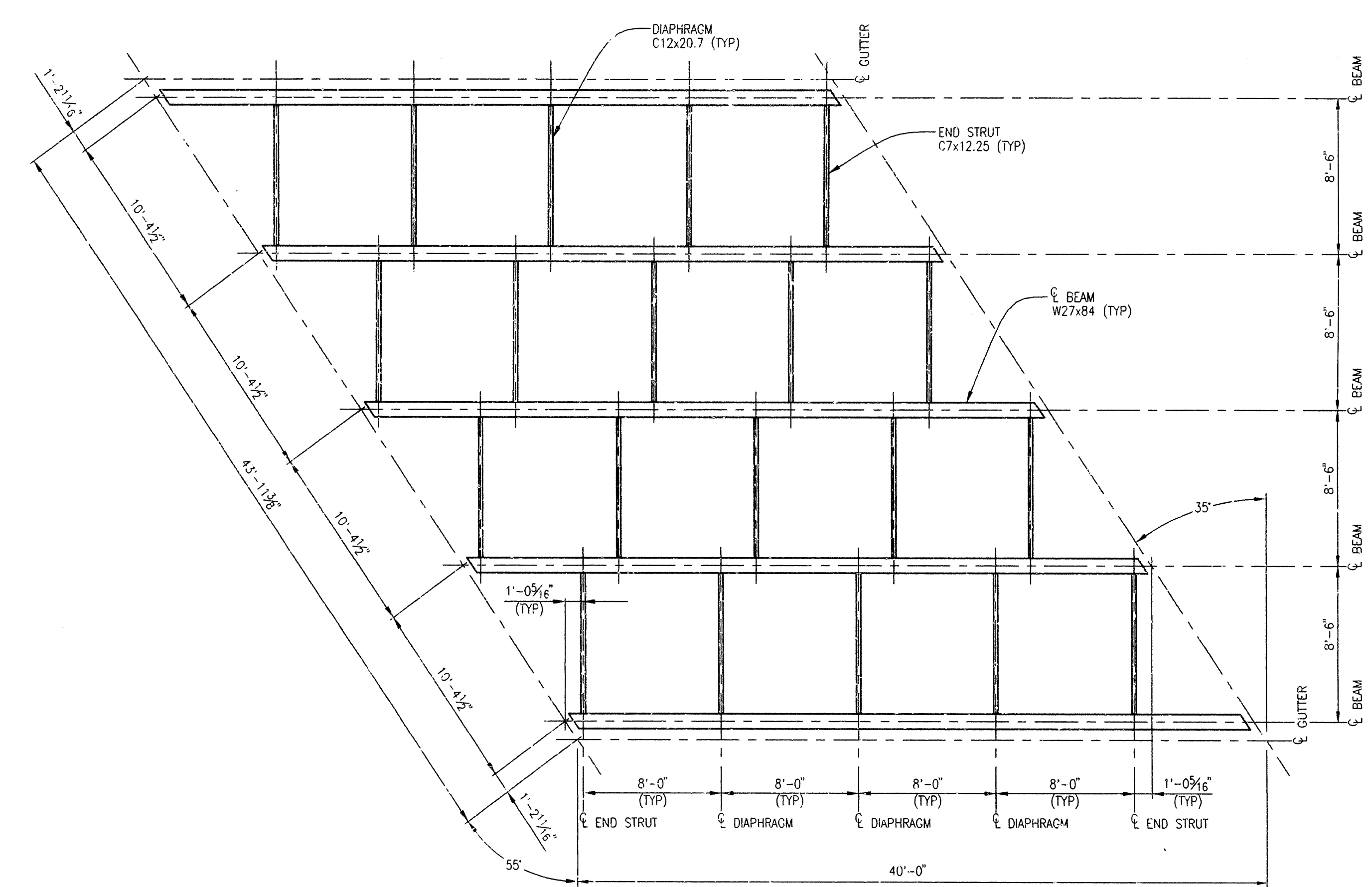
Span Length Load No.	Loading	Location			
		Int. Beam		Ext. Beam	
		1/4 Pt.	1/2 Pt.	1/4 Pt.	1/2 Pt.
4-0-0	1 Bm. & Diaph.	1/100	1/16	1/16	1/100
	2 1 & Slab	1/100	5/8	3/16	1/2
	3 2 & Parapet	1/2	11/16	3/8	9/100
4-0-0	1 Bm. & Diaph.				
	2 1 & Slab				
	3 2 & Parapet				
4-0-0	1 Bm. & Diaph.				
	2 1 & Slab				
	3 2 & Parapet				



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK			
				JOB NO.		SA6030	13	39
				4761		SPAN DETAILS		36398

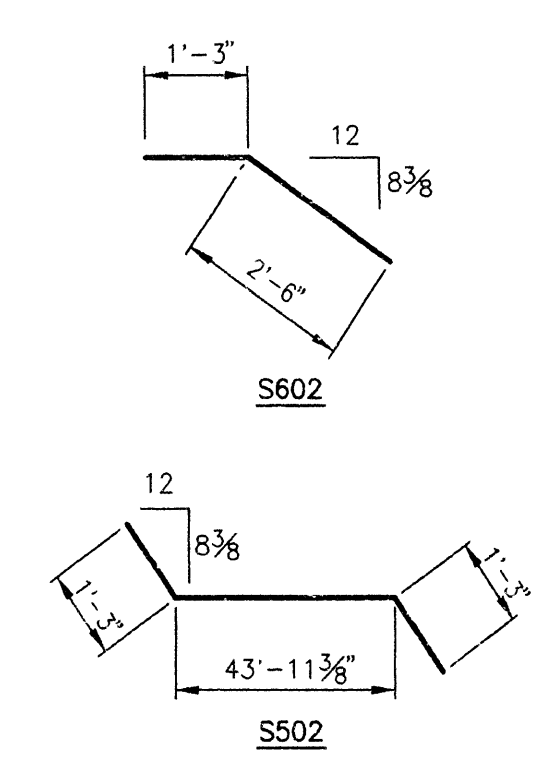


**REINFORCING PLAN**  
SCALE: 3/16" = 1'-0"

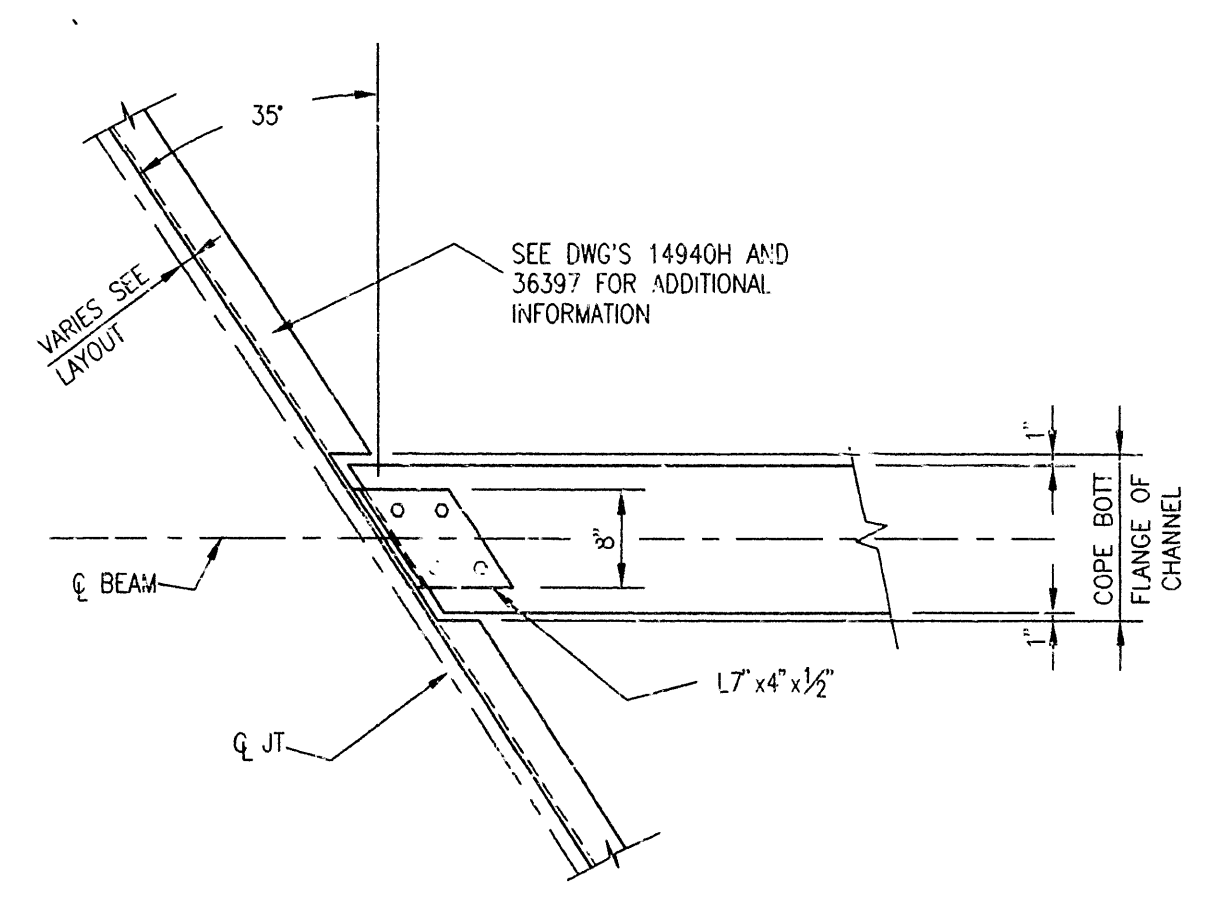


**FRAMING PLAN**  
SCALE: 3/16" = 1'-0"

BAR LIST EACH SPAN				
Mark	NO. REQ'D	Length	Pin Dia.	Bending Diagrams (Dimensions are out to out of bars)
S401	152	20'-7"	STR	
S501	33	38'-4"	STR	
S502	4	46'-4 3/4"	3 3/4"	
S503	28	4'-5"	STR	
S504 - S523	1 EA.	VARIES, 2'-6" TO 36'-4"	STR	
S524 - S542	1 EA.	VARIES, 3'-4" TO 35'-6"	STR	
S543 - S561	1 EA.	VARIES, 3'-0" TO 35'-0"	STR	
S562 - S581	1 EA.	VARIES, 2'-4" TO 36'-0"	STR	
S601	11	38'-4"	STR	
S602	4	3'-9"	4 1/2"	
S603 - S622	1 EA.	VARIES, 2'-6" TO 36'-4"	STR	
S623 - S641	1 EA.	VARIES, 3'-0" TO 35'-0"	STR	

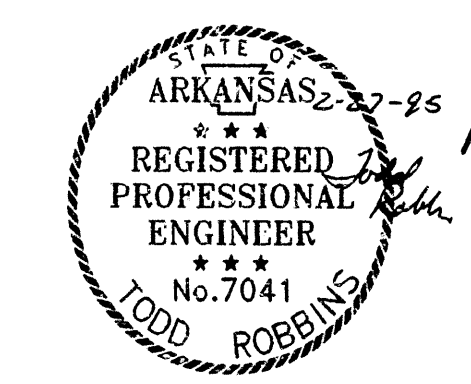


NOTE: DIMENSIONS ARE OUT TO OUT OF BARS.



**CHANNEL CONNECTION DETAIL**  
SCALE: N.T.S.

DETAILS OF  
40'-0" COMPOSITE W-BEAM SPANS  
FOURCHE CREEK STRS. & APPR.  
PULASKI COUNTY  
CO. ROUTE 26 SEC.  
ARKANSAS STATE HIGHWAY COMMISSION



DRAWN BY: C.H. DATE: 22 FEB 95  
CHECKED BY: K.J. DATE: 27 FEB 95  
DESIGNED BY: T.R. DATE: 22 FEB 95  
SCALE: 3/16" = 1'-0"  
FILENAME: 33517.DWG  
BRIDGE NO. 4761 DRAWING NO. 36398